On-site effluent management study Lot 9 in the proposed subdivision of 172 Spring Hill Road, Springhill NSW 2800 Envirowest Consulting Pty Ltd ABN 18 103 955 246 Environmental Geotechnical • 9 Cameron Place, Orange NSW • Tel (02) 6361 4954 •

• 6/72 Corporation Avenue, Bathurst NSW • Tel (02) 6334 3312 •

• Email admin@envirowest.net.au • Web www.envirowest.net.au •

Asbestos Services



Document control

Client Ian Stewart

c\- iPlan Projects 91 Heifer Station Lane Orange NSW 2800

	Grange New 2000				
Rev	Report number	Date	Prepared by	Checked by	Revision details/status
0	R14501e9	28/06/2022	Thilak Suresh MEng Geotechnical Engineer	Eliza Hurst BSc & BNSc Environmental Scientist	

Envirowest Consulting Pty Ltd 9 Cameron Place PO Box 8158 Orange NSW 2800 T 02 6361 4954

6/72 Corporation Avenue Bathurst NSW 2795 T 02 6334 3312

E admin@envirowest.net.au W envirowest.net.au

Copyright © 2022 Envirowest Consulting Pty Ltd. This document is copyright apart from specific uses by the client. No part may be reproduced by any process or persons without the written permission of Envirowest Consulting Pty Ltd. All rights reserved. No liability is accepted for unauthorised use of the report.

1. Summary

i. Summary	
Proposed development and situation	A rural-residential lot requires evaluation for suitability of on-site application of effluent from a proposed new residential dwelling. This report describes the assessment and recommends a suitable effluent treatment and application system.
Investigation	A site assessment and soil assessment were undertaken using the Australian Standard 1547, <i>On-site domestic wastewater management</i> , and the Environment and Health Protection Guidelines, <i>On-site sewage management for single households</i> (1998), Department of Urban Affairs and Planning, as guidelines. Suitable wastewater application systems, sizing and location for the site are recommended.
	The evaluation is based on a proposed dwelling with four bedrooms.
Type of land application and treatment systems	The recommended system is
considered best suited to the site	Surface irrigation with an irrigation area of 488 square metres. Gypsum should be applied to the application area during construction and annually to maintain permeability.
	Secondary wastewater treatment system accredited by NSW Health.
Location	The location of the effluent application area is identified in Appendix 1.
Notes	Construction of the treatment and application systems should be according to AS1547 and Sydney Catchment Authority guidelines, Designing and Installing On-site Wastewater Systems (2019).
	Gypsum should be applied to the application area during construction and annually to maintain permeability.
	Secondary treatment systems require regular maintenance to ensure effective operation. Maintenance scheduling should be undertaken in accordance with manufacturers and NSW Health guidelines.
	The water balance is calculated using full water saving devices such as dual flush toilets (6/3litre water closets), water reduction cycles on dishwashers, aerator faucets fitted to taps, front loader washing machines and water reducing shower heads.
	A maintained grass sward is the recommended vegetation over the irrigation area. Appendix 4 is a checklist of do's and don'ts to ensure correct operation of the wastewater system. Gypsum should be applied every two years to maintain permeability.

2. Introduction

A rural-residential lot requires evaluation for on-site application of effluent from a proposed new residential dwelling. A site and soil assessment were undertaken on 3 June 2022. Boreholes were drilled to 1.0m depth and soil samples collected for analysis. This report describes the site and soil investigation and recommends a suitable effluent treatment and application system.

3. Scope

A site assessment and soil assessment were undertaken using Australian Standard 1547, *On-site domestic wastewater management*, Sydney Catchment Authority (SCA) guidelines, *Designing and Installing On-site Wastewater Systems* (2019) and the Environment and Health Protection Guidelines, *On-site sewage management for single households* (1998), Department of Urban Affairs and Planning, as guidelines. Suitable wastewater application systems, sizing and location for the site are recommended.

4. Site information

Address of site	Lot 9 in the proposed subdivision of 172 Spring Hill Road, Spring Hill NSW 2800		
Local Government	Cabonne Shire Council		
Client	Ian Stewart		
Size	Approximately 3.1ha		
Location, shape, layout	A plan of the relevant areas of the site and proposed effluent application area is described in Appendix 1.		
Photograph(s) attached	Yes		
Intended water supply	Rainwater Reticulated water supply Bore/Groundwater		
Development	New residential dwelling		
Expected wastewater flows	Number of bedrooms – 4		
	Number of persons – 5		
	Flows per person – 120 litres/person		
	Total expected wastewater flow is 600 litres/day		
Flows are calculated using full water saving devices such as dua (6/3 litre water closets), water reduction cycles on dishwashers, a fitted to taps, front loader washing machines and water reducing s			
	Re-calculation of the hydraulic balance and application area is required for dwellings containing a differing number of potential bedrooms.		
Local experience of on-site management systems nearby	All systems are known to work satisfactorily in the locality providing they are adequately designed and maintained.		

Setting	This lot is in a rural setting where the average dwelling density is less than 1 dwelling per 2ha and therefore less than the 1 per 0.4 hectares required for groundwater protection (Geary & Gardner 1996, Land Management for Urban Development, Australian Society of Soil Sciences, Qld).
Current land-use	Grazing
Climate	Summers are warm to hot and winters are cold with little or no effective evaporation. Rainfall is distributed evenly throughout the year with an average annual rainfall of 832mm and pan evaporation of 1,335mm (Bureau of Meteorology, Orange).

5. Site assessment

Work undertaken	Details
Date	03 June 2022
Details	Site inspection, borehole drilling to 1.0m depth or refusal, soil sampling and analysis
Weather on day and preceding week	Overcast, windy, >25mm rain in preceding week

Site feature	Assessment	Limitation
Vegetation	Grasses, plantain, clover, broadleaved weeds	Minor
Flood potential: 1 in 20 year 1 in 100 year	Nil Nil	Minor
Exposure Site aspect Shelter belts Topographical feature or structure	Moderate West Nil Nil	Moderate
Slope	0-1% in application area	Minor
Landform	Mid-slope	Minor
Run-on and seepage: Comment	Run-on and sub-surface seepage is expected to be low. Diversion banks may be required to divert flows from upslope sources.	Minor
Erosion potential: Erodibility and erosion hazard	The topsoil and subsoil have a low erodibility. Erosion hazard is low and is reduced when vegetated.	Minor

Site drainage	Moderate. Heavily mottled grey clays identified from 800mm.	Moderate
Fill	Nil	Minor
Groundwater: Level of protection Bores and wells in the area and their purpose	Low One groundwater bore is located within 100m of the recommended application area. The bore (GW050844) is located approximately 60m downslope east of the recommended application area. The standing water level (SWL) of the bore is 1.9m. Water bearing zones (WBZ) were from 10.7m to 12.0m. Licensed use of four groundwater bores are located within 500m of the recommended application area. The Sydney Catchment Authority recommends a draw-down analysis using an appropriate methodology such as Cromer et. al 2001 be undertaken when an effluent application area is located within 100m of a bore used for domestic consumption. The Cromer methodology was applied to the bore as calculated in Appendix 5. Bore characteristics were conservatively estimated from data of existing bores in the locality. A minimum buffer distance of 15m is required around the bore. This buffer distance is available. No impact on groundwater is expected from the application of effluent to the site.	Minor
Surface water: Permanent waters, streams, lakes (Recommended buffer distance 100m)	Nil	Minor
Other waters, intermittent waterways (Recommended buffer distance 40m)	Two dams approximately 175m and 215m northeast and a drainage line 175m northeast from the recommended application area	
Buffer distances from recommended application area to: Boundary premises (Recommended buffer distance 6-12m) Swimming pools (Recommended buffer distance 6-12m) Buildings (Recommended buffer distance 6-12m)	>6m Nil >6m	Minor

Area required for application system(s): Area available (including buffers):	63m² minimum area required for trench systems. 488m² minimum area required for irrigation systems. Potential application area of greater than 2,000m² available (Appendix 1).	Minor
Surface rocks, rock outcrops	Surface rocks and rock outcrops were scattered across the site.	Moderate
Geology	The site is within the Spring Hill Soil Landscape. This soil landscape occupies a large area south and south-east of Orange. Krasnozems are dominant soils. Yellow Podzolic soils occur on the lower slopes with Yellow Solodic soils in drainage lines. The geological unit is tertiary volcanics from Mount Canobolas. Parent rock consists of basalt flows which are separated by volcanic ash forming layers of clay and slate. Parent material consists of in situ materials or colluvium derived from Tertiary volcanics (eSPADE v2.2).	Minor
Environmental concerns: Native plants intolerant of phosphorous	Nil	Minor
High water table	Nil	
Water way/wetland	None nearby	
Community water storage	None nearby	
Site stability: Is expert assessment necessary	No, not expected to affect system performance	Minor

6. Soil assessment

Soil was assessed on site on 3 June 2022 by borehole construction to a depth of 1.0 metres or drill refusal with a ute mounted hydraulic corer.

The soil profile was described, and representative samples collected for the determination of physical and chemical properties. Soil physical property measurements undertaken included Emerson aggregate test (dispersion description), texture, colour, pH, and salinity (ECe). The laboratory tests for physical properties were undertaken by Envirowest Testing Services and results are presented in the following table.

Depth (mm)	Description	Sampled (mm)	Texture group	Moisture	Emerson aggregate test*	pH (1:5 water)	ECe dS/m
T () . ()		Ø					
Test hole 1					_		
0-200	Greyish brown clay loam with trace coarse sand	100	CL	М	5	6.2	0.26
200-700	Yellowish brown silty clay with trace coarse sand	600	ZC	M	5	6.2	0.15
700-1000	Yellowish grey silty clay with abundant fine to	900	ZC	W	5	6.3	0.02
	medium ironstone gravels and heavily mottled						
	grey clay						
1000	End of hole at investigation depth						
Test hole 2	•	•		•			•
0-300	Greyish brown clay loam with fine to medium	-	-	-	-	-	-
	gravels						
300	End of hole, refusal on rock						
Test hole 3							
0-200	Greyish brown clay loam with fine to coarse sand	100	ZC	M	5	6.3	0.52
200-400	Dark yellowish brown silty clay with fine to medium ironstone gravels	-	ZC	М	-	-	-
400-800	Greyish brown silty clay with fine ironstone	600	ZC	М	5	6.5	0.23
100 000	gravels	550	20	.,,,		0.0	3.20
800-1000	Greyish brown silty clay with fine to medium	900	ZC	w	3	6.7	0.08
333 1000	ironstone gravels and heavily mottled grey clay	550		'*		0.7	0.00
1000	End of hole at investigation depth						
	/		<u> </u>	·	L		L

M=Moist, D=Dry, W=Wet *1= highly dispersive (slakes, complete dispersion), 2= moderately dispersive (slakes, some dispersion), 3= slightly dispersive (slakes, some dispersion after remoulding), 4= non-dispersive (slakes, carbonate or gypsum present), 5= non-dispersive (slakes, dispersion in shaken suspension) 6= non-dispersive (slakes, flocculates in shaken suspension), 7= non-dispersive (no slaking, swells in water), 8= non-dispersive (no slaking, does not swell in water).

Site feature	Assessment	Limitation
Depth to bedrock	Greater than 300mm in recommended application area (600mm below application base recommended)	Major
Depth to high water table	Greater than 1,000mm in recommended application area (600mm below application base recommended)	Minor
Coarse fragments	Gravels identified throughout the soil profile	Minor
Bulk density	Good (estimated)	Minor
рН	Satisfactory (4.5-8.5 optimum range)	Minor
Salinity	Non-saline (<4.0 dS/m desirable threshold)	Minor
Phosphorus sorption capacity (SCA, 2012)	6,500 kg/ha estimated	Minor
Nutrient balance	Water is not expected to move off site, nutrients will be utilised by the vegetation and stored in the soil. The subsoil is a moderately drained silty clay that will immobilise moderate quantities of nitrogen (in ammonium and organic forms) as derived from primary treatment systems.	Minor
Cation exchange capacity	Moderate (estimated). Will provide adequate retention of nutrients for plant growth.	Minor
Dispersiveness (Emerson aggregate test)	Non-dispersive clay loam topsoil over a non-dispersive to slightly dispersive silty clay subsoil. Regular application of gypsum is recommended at the rate of 1kg per square metre of application area.	Minor

Soil structure	Strongly structured	Minor
Soil texture and permeability category	Clay loam CL (100mm)	Minor
	Light clay LC (600mm)	

7. System selection

7.1 Estimation of land application areas from hydraulic loadings

Rainfall water balance and land application area calculations are presented in Appendix 3 and are summarised in the following table. Design flow rates for the dwelling are 600L/day based on the use of water saving features. Wet weather storage areas included in the water balance utilise the storage capacity of the soil. The design application rate was determined from Tables L1, M1, N1 in AS1547 using the permeability classification of the subsoil.

Factors Affecting Design Loading and Sizing		Design application rate (AS1547) (mm/day)	Size required for effluent application	
Hydraulic loa	ading for different application systems			
- Surface/s	ub-surface irrigation	3	488m ²	
- Absorption/ Evapotranspiration trench		8	63m²	
Notes		ading will provide for leaching of salts out of the root zone and prevent the soil odic. The proposed infiltration rates will protect the catchment against off-site nt.		

7.2 Centralised sewerage systems

Consideration of connection to a centralised sewerage system	
Approximate distance to nearest feasible connection:	>500m
Potential for future connection to centralised sewerage:	high / medium / low / already connected
Potential for future connection to reticulated water:	high / medium / low / already connected

7.3 Suitability of application systems

Application system	Treatment system	Site limitations of the application system	Modifications to mitigate constraints	Suitability
Absorption system	Septic tank	Moderately drained subsoil Surface rock and rock outcrops Shallow subsurface rock from 300mm	Nil	No
Evapotranspiration system	Septic tank	Moderately drained subsoil Surface rock and rock outcrops Shallow subsurface rock from 300mm	Nil	No

Surface irrigation	Secondary	Slightly dispersive topsoil	Regular application of gypsum	Yes
		Surface rock and rock outcrops	Ensure wastewater is applied to the soil	
		Shading from trees reducing exposure	Lop branches shading application area	
Sub-surface irrigation	Secondary	Slightly dispersive topsoil	Regular application of gypsum	Yes
		Surface rock and rock outcrops	Ensure wastewater is applied to the soil	
		Shading from trees reducing exposure	Lop branches shading application area	

7.4 System recommendation		
Type of land application and treatment systems considered best suited to the site	 Surface irrigation with an irrigation area of 488 square metres. Gypsum should be applied to the application area during construction and annually to maintain permeability. Secondary wastewater treatment system accredited by NSW Health. 	
Location	The location of the effluent application area is identified in Appendix 1.	
Notes	Construction of the treatment and application systems should be according to AS1547 and Sydney Catchment Authority guidelines, Designing and Installing On-site Wastewater Systems (2019). Gypsum should be applied to the application area during construction and annually to maintain permeability. Secondary treatment systems require regular maintenance to ensure effective operation. Maintenance scheduling should be undertaken in accordance with manufacturers and NSW Health guidelines. The water balance is calculated using full water saving devices such as dual flush toilets (6/3litre water closets), water reduction cycles on dishwashers, aerator faucets fitted to taps, front loader washing machines and water reducing shower heads. A maintained grass sward is the recommended vegetation over the irrigation area. Appendix 4 is a checklist of do's and don'ts to ensure correct operation of the wastewater system. Gypsum should be applied every two years to maintain permeability.	

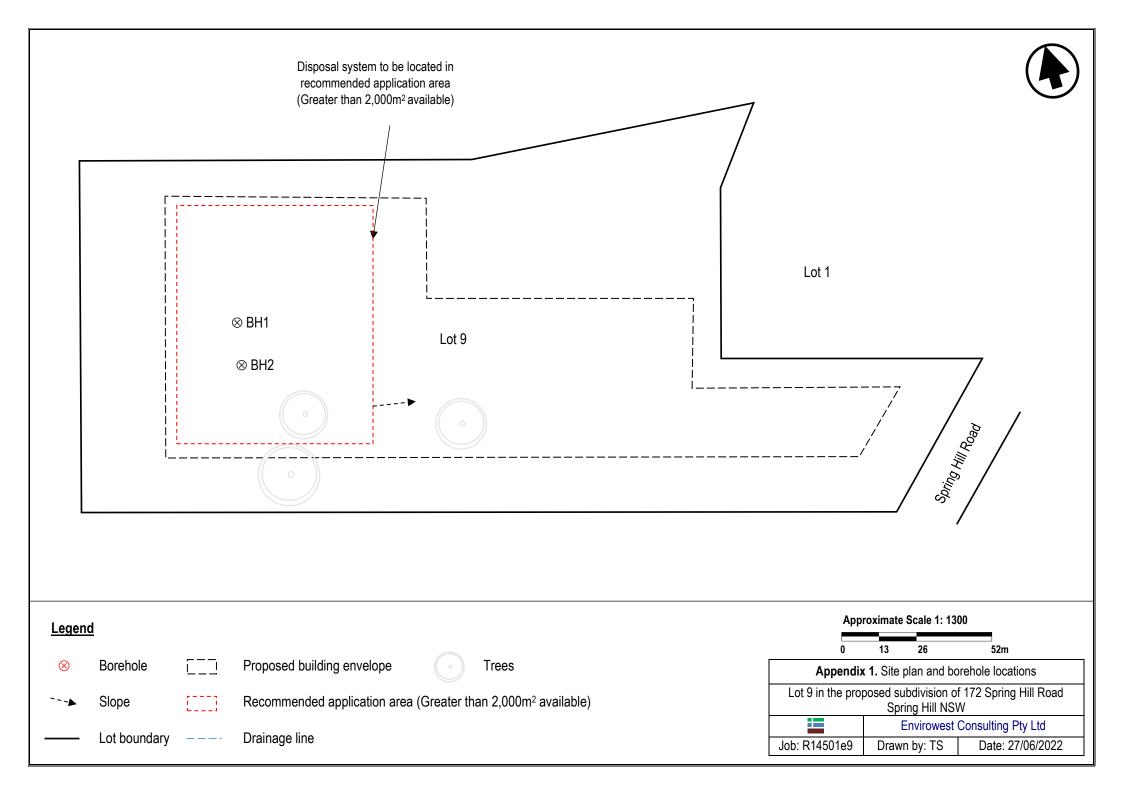
8. General comments

Are there any specific environmental constraints?	Wastewater should be evenly applied over the application area.
Are there any specific health constraints?	Restrict access to people and stock as recommended in AS1547 and summarised in Appendix 4.
Any other comments?	The topsoil is capable of supporting plant growth that will optimise evapotranspiration and wastewater usage.

9. Report limitations and intellectual property

This report has been prepared for the use of the client to achieve the objectives given the clients requirements. The Australian Standard 1547, *On-site domestic wastewater management*, and the Environment and Health Protection Guidelines, *On-site sewage management for single households* (1998) Department of Urban Affairs and Planning, have been used as guidelines in this report. Where system limitations or uncertainties are known, they are identified in the report. No liability can be accepted for failure to identify conditions or issues which arise in the future and which could not reasonably have been predicted using the scope of the investigation and the information obtained. No guarantee can be made that the wastewater system will achieve all performance criteria because of operational factors and the inherent variable and unpredictable nature of the soil. All components of the wastewater system have a limited life.

This report including data contained, its findings and conclusions remain the intellectual property of Envirowest Consulting Pty Ltd. A licence to use the report for the specific purpose identified is granted after full payment for the services involved in preparation of the report. This report should not be used by persons or for purposes other than those stated, and not reproduced without the permission of Envirowest Consulting Pty Ltd.



Appendix 2. Photograph of the recommended application area



Looking northeast over the recommended application area

Appendix 3a. Monthly water balance to determine the wastewater application area required for Irrigation systems Design wastewater flow L/day 600 120 L/person/day 5 persons Design percolation rate R 21 3 mm/day mm/wk Land area L m2 130 ΕP Effective precipitation 0.9 (10% runoff) Parameter Symbol Formula Units Jan Feb Mar May Oct Dec total Apr Jun Jul Aug Sep Nov days in month D days 31 28 31 30 31 30 31 31 30 31 30 31 365 45.9 47.7 79.7 86 76.2 81.9 83.6 70.6 832.2 Ρ 61.3 86.4 36.2 76.3 Precipitation mm/month 216 157 137 94 51 41 38 51 81 114 152 203 1335 Е Evaporation mm/month Crop factor С 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 10.8 Inputs Effective Precipitation ΕP mm/month 55.17 77.76 41.31 32.58 42.93 71.73 68.67 77.4 68.58 73.71 75.24 63.54 749 Effluent irrigation W QXD/L 143.1 129.2 143.1 138.5 143.1 138.5 143.1 143.1 138.5 143.1 138.5 143.1 1685 mm/month 211.7 Inputs P+W mm/month 198.2 207.0 184.4 171.0 186.0 210.2 220.5 207.0 216.8 213.7 206.6 2433 **Outputs** ΕT ExC 123.3 1202 Evapotranspiration 194.4 141.3 84.6 45.9 36.9 34.2 45.9 72.9 102.6 136.8 182.7 mm/month В Percolation R/7xD 93.0 93.0 90.0 93.0 90.0 93.0 mm/month 93.0 84.0 90.0 93.0 90.0 93.0 1095 ET+B 287.4 Outputs mm/month 225.3 216.3 174.6 138.9 126.9 127.2 138.9 162.9 195.6 226.8 275.7 2297 Storage S (EP+W)-(ET+B) mm/month -89.2 -18.3 -31.9 -3.6 47.1 83.3 84.5 81.6 44.1 21.2 -13.1 -69.1 Cumulative storage M mm 0.0 0.0 0.0 0.0 47.1 130.4 214.9 296.5 340.7 361.9 348.8 279.7 Storage ٧ 361.9 largest M mm 372.0 Soil storage mm Storage -10.1 required water holding capacity depth (mm) Totals(mm) mm VxL/1000 m^3 -1.3 Topsoil 34% 200 68 Subsoil 38% 800 304 Irrigation area m^2 130 372

m2

Appendix 3b. Estimation area requirement from organic matter and nutrient balances (Irrigation systems)

Estimated effluent flow

(Q)

600

L/day

Soil depth

1

Organic matter balance

BOD (C) 20 mg/L L/da treated wastewater flow rate (Q) 600 y 300

critical loading rate of BOD (Lx) 0 mg/m²/day

land area required (A) 4.0 m²

Nitrogen balance

nutrient concentration 37 mg/L treated wastewater flow L/da rate 600 y

critical loading rate of nutrient 50 mg/m²/day

land area required (A) 444 m²

Determination of nitrogen critical loading rate

Nitrogen load (kg/year) 8.1 kg/year Loss 20% denitrification 6.5 kg/year

Load to soil assumed irr. 44
Load to soil 146.0 kg/ha/year area 4

Vegetation usage 200.0 kg/ha/year from table

Residual (potential

leaching) -54.0 kg/ha/year

Typical nitrogen uptake (Myers et al. 1984)Pastures300 kg/ha/year82 mg/m2/dayPine350 kg/ha/year96 mg/m2/dayEucalypts180 kg/ha/year49 mg/m2/day

Phosphorus balance

Puptake=

Phosphorus sorption capacity per metre= 6,50

O kg/ha 6,50

Phosphorus sorption capacity of profile= 0 kg/ha

Soil factor 0.33

Critical loading= mg/m²/day
P concentration*= 12 mg/L

P adsorbed= phosphorus sorption capacity x soil factor

2145

 $\begin{array}{ccc} \text{0.2145} & & \text{kg/m}^2 \\ \text{critical loading x} & & \text{year} \\ \text{days/year x} & & 50 & \text{s} \\ \end{array}$

54750

0.0548 kg/m²

year
Pgenerated= total phosphorus concentration x wastewater volume in 50 s

131400000

131 kg Pgenerated / (Padsorbed + Puptake)

Land area required 488.0 m²

Appendix 4. Checklist for effective management of wastewater systems

Domestic wastewater system

DO

- Check household products for suitability of use with a septic tank or secondary treatment tank.
- Conserve water, prolonged period of high water use can lead to application area failure.

 For optimum operation, avoid daily and weekly surges in water flows. Spas are not recommended.
- Scrape cooking dishes and plates prior to washing to reduce solid load.
- Maintain the system with regular servicing as per the manufacturer's instructions.

DON'T

• Dispose of excessive solid material, fats, lint or large water volumes into drains.

Land application area

- Construct and maintain diversion drains around the top-side of the application area to divert surface water.
- The application area should be a grassed area, which is maintained at 10-30cm height.
- The area around the perimeter can be planted with small shrubs to aid transpiration of the wastewater.
- Ensure run-off from the roof or driveway is directed away from the application area.
- Periodic application of gypsum may be necessary to maintain the absorptive capacity of the soil.
- **Do not** erect any structures or paths on the land application area.
- **Do not** graze animals on the land application area.
- **Do not** drive over the land application area.
- **Do not** plant large trees that shade the land application area thereby reducing transpiration of water.
- **Do not** let children or pets play on the land application area.
- Do not extract untreated groundwater for potable use.

Appendix 5. Buffer distances for bores

The recommended buffer distance for on-site effluent management systems to groundwater wells is 100m. One bore is located 60m east and downslope from the recommended application area. The size of the buffer distance from the bores can be reduced by determining the separation distance required between the bore and an on-site application system.

The separation distance is the distance required between a bore and a land application system to prevent contamination of the bore with effluent that may enter the bore. The separation distance is determined from the radius of influence of a bore plus the setback distance.

The radius of influence of a bore can be calculated from the aquifer and bore hydraulic characteristics as an application of the viral die-off method of Cromer *et al.* (2004). The viral die-off method estimates the time required for viruses in the contaminated water to be inactivated (reduced to acceptable number by natural mortality processes) as they move down gradient in the groundwater. The distance travelled during the travel time is the setback distance. Darcy's law is used to estimate the travel time.

The model for estimating the setback distance is:

```
dg = (t-dv.P/K) / (P/K. i)
```

where:

d_g = setback distance (m)

t = time (days)

 d_v = vertical distance to water table (m)

P = porosity of fraction (decimal)

K = hydraulic conductivity (m/day)

i = groundwater gradient (fraction)

The model for estimating the radius of influence of a water bore is:

 $r = 1.5[(KHt/S)^{0.5}]$ which is reasonably valid for $t=Kt/SH \ge 1$

where:

r= radius of influence

K= aquifer permeability (m/day)

H= initial thickness of the water (m) in the fully penetrating bore

t= time of pumping (t, days)

S= specific yield (S fraction, dimensionless)

A land application system should not be located within the maximum radius of influence of a bore. Additionally, the appropriate separation distance is the radius of influence of the bore plus the setback distance for viral die-off when application systems are located up gradient of the bore. The application system will be located up gradient of the bore approximately 60m east and downslope of the application area therefore the radius of influence plus the calculated setback distance is the appropriate buffer distance.

No impact from the application of effluent is expected on the bore. Bores surrounding the site are confined aguifers.

An assessment of potential impacts on confined aquifers was undertaken by modelling. The viral die-off method of Cromer *et al.* (2004) was used to calculate the radius of influence and subsequently the minimum separation distance required to the well.

Viral die off time was estimated to be a reduction in order of magnitude of 3 at a groundwater temperature of 9.6°C equivalent to 100 days. This is expected to be a conservative estimate in viral die-off.

The model parameters for estimating the radius of influence of the water bore were:

```
K= aquifer permeability (m/day) = 0.5
H= initial thickness of the water (m) in the fully-penetrating bore = 1.30m
t= time of pumping (t, days) = 100
S= specific yield (S fraction, dimensionless) = 1.260
```

The radius of influence was subsequently calculated to be 3 metres.

The model parameters for measuring the setback distance of the groundwater bore were:

```
t= time (days) = 100

d_v= vertical distance to water table (m) = 1.30

p= porosity of fraction (decimal) = 0.2

k= Hydraulic conductivity (m/day) = 0.5

i= groundwater gradient (fraction) = 0.02
```

The calculated setback distance was subsequently calculated to be 4 metres.

The appropriate separation distance is the radius of influence plus the calculated setback distance, which is subsequently calculated to be 7m.

A conservative buffer distance of 15 metres is therefore required around the bore which is the minimum recommended by the Australian Standard.